

Memorandum

To: SCL File No: 423022-m-e-c004.docx

Attention: WBOPDC
From: Stephen Bos
Date: 31 May 2023

Subject: Tinex Group Ltd – 245 Te Puna Station Road –

Existing Site Usage - Stormwater Queries

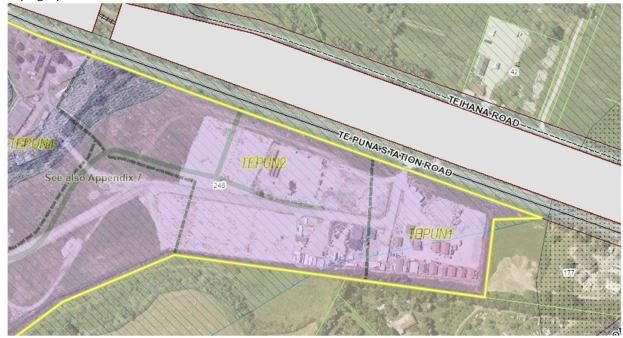
Further to the WBOPDC RFI and meeting the following response is provided in terms of the relevant stormwater queries:

STORMWATER MANAGEMENT

- 1. The stormwater assessment provided is lacking in technical evidence. Please provide:
 - a) calculations to demonstrate that the stormwater impact from the current users on the overall catchment is minimal.
 - b) Topographic Plans showing the existing land contours/spot heights, and overland flow paths.
 - c) Additional information that further qualifies the statement that "from the discharge location, the flows disperse across the full site area such that mitigation of the additional flow is considered to occur prior to exiting the individual yard site".

Stormwater Management Response

The site is currently zoned Industrial, and the site lies within the Te Puna Industrial Park, with layout as below. A detailed site survey of the area is attached to this response that shows the layout and topographical details of the site.



Also now included is the identification of the site falls, with noted bunding to the south, east and part north. The existing overland flow paths are shown and there is a series of drains, minor culverts and discharge points that collect the site flow and run this to the existing main drain along the south side of Te Puna Station Road.

The site has been formed and used in its current state for a number of years, as per the 2019 aerial noted below



Under the WBOPDC District Plan for Industrial land, storage is a permitted activity subject to Structure Plan requirements being met.

The baseline assessment for all discharges from the site is acceptance that the site is zoned Industrial and is fully metalled, such that stormwater can be discharged off the site from a cleared and paved site.

It is acknowledged that there are a number of issues associated with the greater catchment and stormwater discharges within the catchments are being reviewed and specifically addressed by the stormwater modelling consultant (Dr Steve Joynes). The intention of this response is <u>not</u> to address the larger catchment issues but to confirm that the ongoing internal site activities have no greater effect than the baseline permitted activity status, (ie the effect of the current off-site flows from the site generated activities are not creating a greater impact that what would be permitted under a permitted activity).

From the larger catchment modelling results the physical location of the site (towards the lower end

of the catchment) and the singular effect of the runoff generated from the base site is not considered to be the primary cause of flooding or noted stormwater issues within the catchment. In addition, further modelling works are currently underway to identify potential remedial works that result in the upstream water levels being lowered to ensure the level of effects on all parties are minimised and remain as the Structure Plan Baseline Intends.

With regards to the current site activities we confirm as follows:

The overall land area of the subject site is noted as 12.2 ha, of which currently 3.8 ha is metalled. The balance of the site is currently in grass and pasture with access tracks across the site.

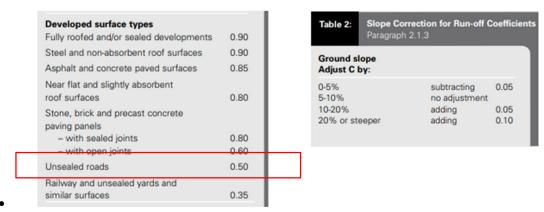
Within the 3.85 ha, site activities are underway that generally involve either open stockpiling of material, temporary storage of materials including fibreglass swimming pools, containers and the like and temporary storage of relocatable houses.

No specifically designed drainage systems has been installed within the site (to service the site storage areas), although there is a series of water table drains and minor culverts crossings throughout the site with allow vehicle access to the storage yard areas. The layout of the existing drainage pipes and water table drains is shown on the attached plan. All internal site flows generally pass to the north and either discharge through pipes installed through the main northern site bund, or around the main vehicle entrance where the water tables are formed to drop into the existing main road drain.

Site operations are such that all roof and hard surface area discharge directly onto the ground directly adjacent the particular storage unit/house roof, and from this point the water tracks as surface water to reach the existing drainage features.

A previous statement has been made that site mitigation of the existing site flow is provided by utilisation of the flat grades across site and the sheet flow effect of discharging to ground as opposed to conveying the flows in a piped system and discharging this directly to the outfall at the boundary. This is further expanded as:

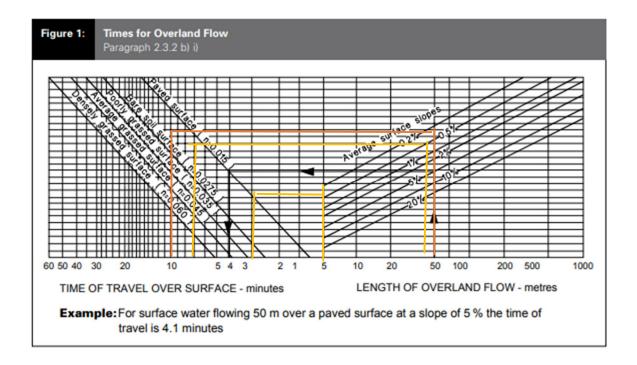
- Through past site activities, in isolation and not considering the filling undertaken to raise the site, the site area currently being used for industrial storage activities has been metalled and is predominantly 'flat'. Grades in the order of 0.25% are noted (as per the attached survey plan).
- The expected runoff coefficient from the site is taken as 0.5 as per NZBC E1 / VM 1 tables 1 and 2



- The site usage is noted as storage with all activities discharging onto ground where they flow overland to a drainage outfall point.
- Clause 2.3 of NZBC E1 notes that the time of concentration for stormwater to enter the system (in this case the site outfall) is taken as $T_{entry} + T_{flow}$
- Clause 2.3.2 a) is identified as 5 minutes as per the description below
 The time of entry t_a:
 - a) Where the catchment area has a well defined and regularly repeated pattern for directing the *surface water* to the *drain* or open channel, the time of entry may be taken as:
 - t_e = 5 minutes for commercial or industrial areas where greater than 50% of the surface of the catchment area feeding the *drain* or open channel consists of roofed, asphalt, concrete, paved or metalled surfaces.

As per the attached marked up plan the time of overland flow is considered to vary from the areas closest to the outfall to those furthest away. It is considered that the existing seal yard and flow path runs have been set up to match actual site activities, however these are also considered to be reflective of the site should no activities be undertaken and the yard be sealed only. It is reasonable to assume that water table drains would be formed across the site to enable the collection and discharge of surface water landing on the site.

Based on a generalised distance of 50m run from the centre of the site to the nearest drain outlet the minimum time of flow is considered to be around 10 minutes as per the extract from the graph below.



The baseline time of concentration is therefore considered to be in the order of 15 minutes. Based on a reassessment of 5m of paved flow (roof or hard paved object) and then 45m of metalled surface flow the reviewed TOC would be 5 + 3 + 7 = 15 minutes or equivalent to the baseline.

Further an overall check of the runoff coefficient considered that compared to the full site development at a paved coefficient of 0.5 vs the current site usage with a minor component at 0.9 runoff the gross runoff factor does not change and the effective increase is 0.04% which is less than minor.

In terms of WBOPDC policy for stormwater improvement, we note that the works applied for under this consent are the temporary works to enable the existing site to continue as current. There is a full consent application underway to provide for the final site development and this addresses stormwater control and mitigation with the introduction of swales, ponds and filter systems that provide both stormwater treatment and retention. The effects of the current application are neither mitigated or exacerbated and are requested to continue while the new application is under process.

In summary as per the methodology outlined above, as there is no fully piped stormwater system within the site, all runoff from the site users (stored buildings / pools and vehicles etc) is considered to outflow onto ground and then via overland flow to open water table drains to then discharge at the site boundary in the locations as noted. Based on the permitted baseline of a metalled surface for the industrial site there is no expectation of a faster or greater runoff with the site activities as currently occurring.

We trust this suitably responds to the queries raised. Please contact the undersigned if you have any further queries.

Yours faithfully STRATUM CONSULTANTS LTD

Stephen Bos

CPEng, CMEngNZ, BE(hons), NZCE (civil/Struct)

Job Title Site Address

Job No.

Tinex Group

423022

City Te Puna

245 Te Puna Station Road No of Pages

> Ву SB

Date May 23

Page

TGA REV 2 - 07/07/2010



Stormwater Catchment Data

 m^2 Existing Site Area = 122126

Existing runoff coeff 0.5

Rainfall Intensities mm/hr HIRDS V4

Return		Durat	ion (mir	2	6	12	
Period	10	20	30	60	120	360	720
10	124.0	97.0	82.5	60.7	42.6	22.1	13.9
50	180.0	139.0	118.0	86.5	60.4	31.2	19.4
100	206.0	158.0	135.0	98.6	68.7	35.4	22.0

Existing Site Discharge (Q = CIA)

0 0 7										
Return		Durat	tion (mir	nutes)						
Period	10	20	30	60	120	360	720			
10	2103	1645	1399	1030	723	375	236			
50	3053	2358	2002	1467	1025	529	329			
100	3494	2680	2290	1672	1165	600	373			

Developed Site Discharge (Q = CIA) I/s

Return		Dura	ation (minu				
Period	10	20	30	60	120	360	720
10	2104	1646	1400	1030	723	375	236
50	3054	2359	2002	1468	1025	529	329
100	3495	2681	2291	1673	1166	601	373

% Change 0.04%

Revised runoff coeff	Runoff

coeff Total 119876 0.5

59938 Site Full (less hardstand change) hardstand change 1275 0.9 1147.5

> TOTAL 61085.5

Average runoff coeff = 0.500 HIRDS V4 Intensity-Duration-Frequency Results

Sitename: Te Puna Station Road Coordinate system: WGS84

Longitude: 176.0859 Latitude: -37.6855

DDF Model Parameter: c d e f g h i

Values: -0.00449 0.540617 -0.03861 0 0.276136 -0.01076 3.247881

Example: Duration (h ARI (yrs) x y Rainfall Rate (mm/hr)

24 100 3.178054 4.600149 10.75094

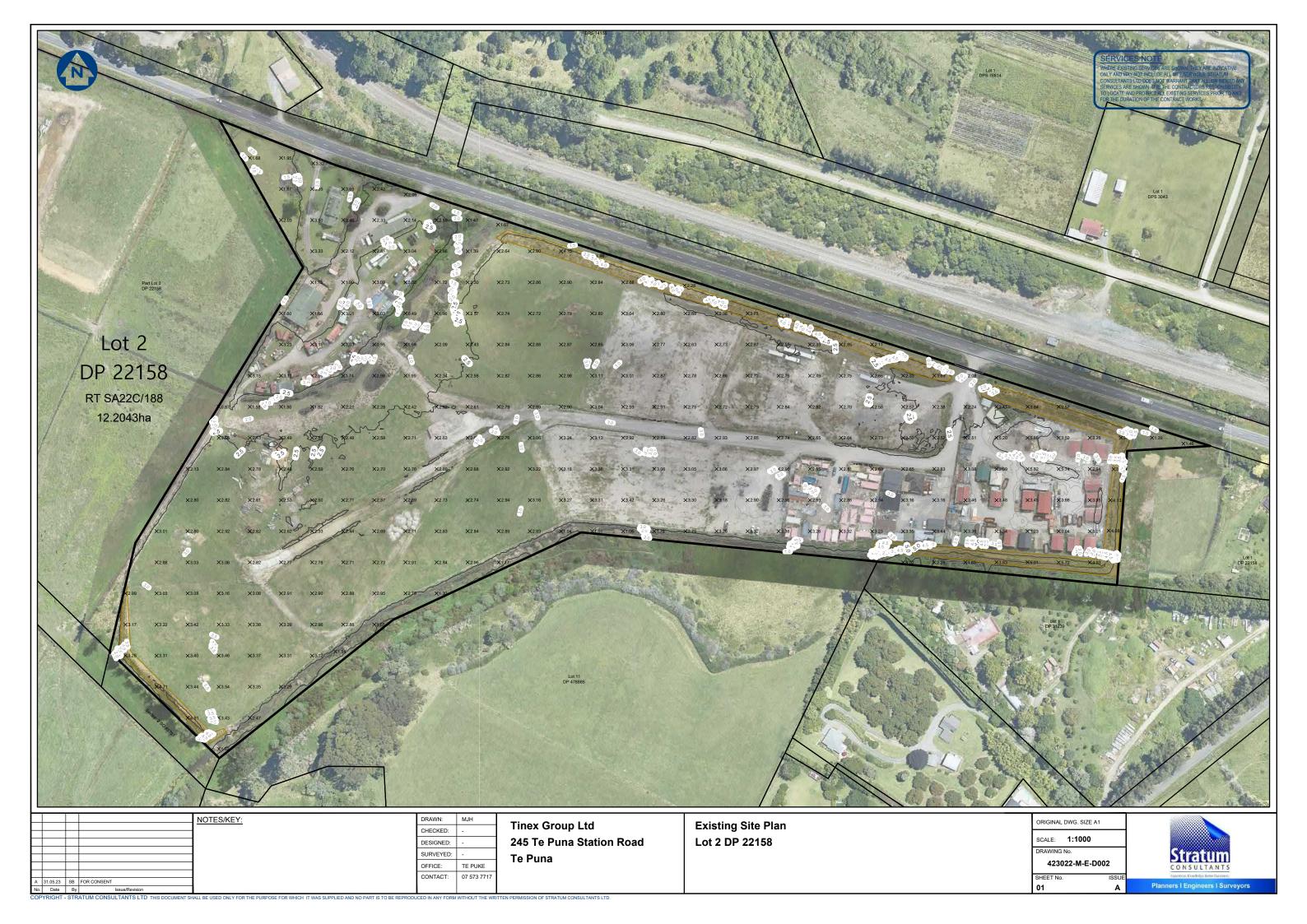
Rainfall intensities (mm/hr) :: Historical Data

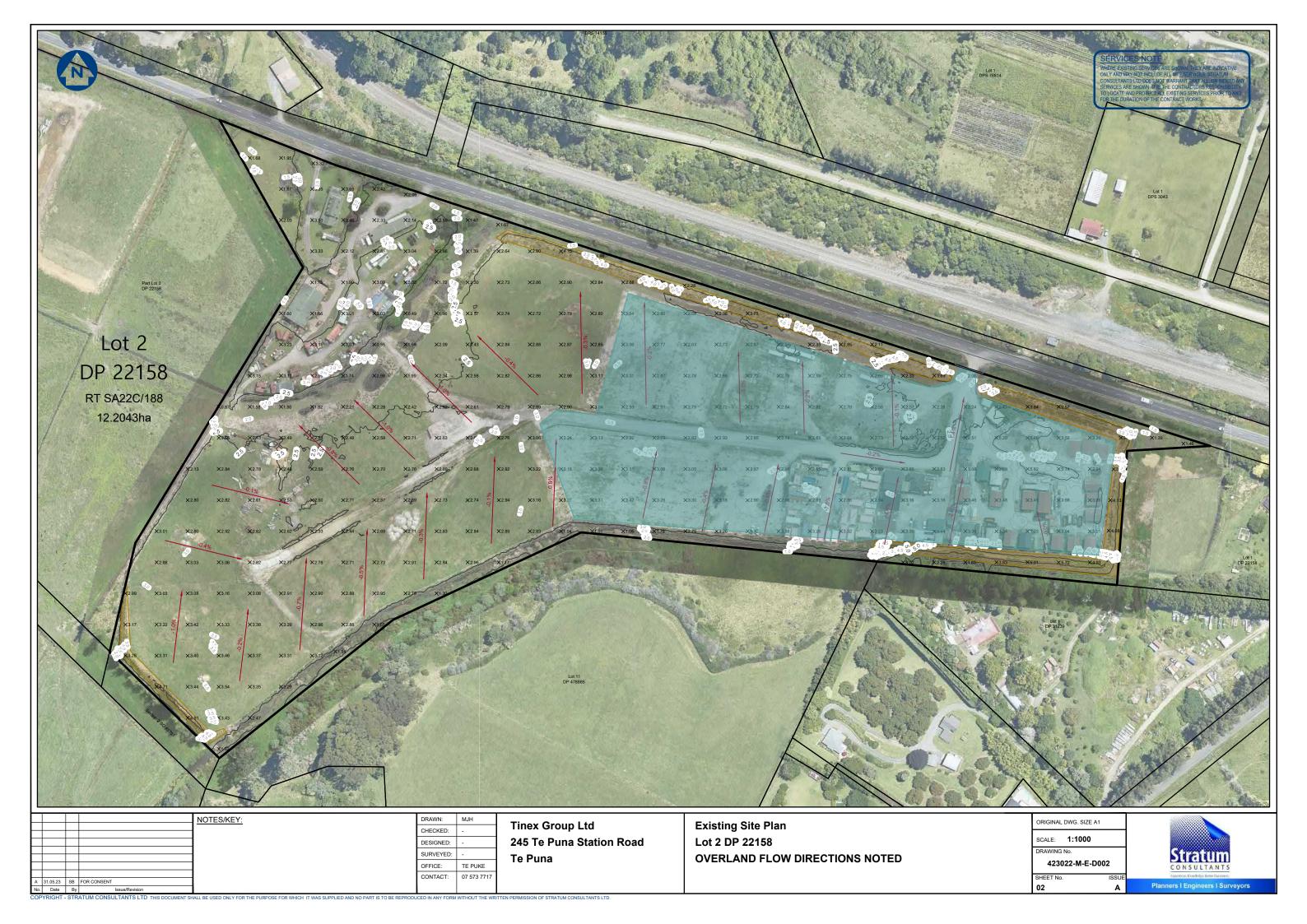
ARI	AEP	10m	20m	30m	1h	2h	6h	12h	24h	48h	72h	96h	120h
1.58	0.633	51.8	40.7	34.7	25.7	18.4	9.98	6.47	4.04	2.44	1.78	1.41	1.18
2	0.5	57.4	45	38.4	28.4	20.3	11	7.13	4.45	2.68	1.95	1.55	1.29
5	0.2	77.4	60.5	51.5	38	27	14.6	9.4	5.85	3.51	2.56	2.03	1.68
10	0.1	92.9	72.5	61.7	45.4	32.2	17.3	11.1	6.91	4.13	3.01	2.38	1.98
20	0.05	110	85.3	72.4	53.2	37.6	20.1	12.9	8.01	4.78	3.47	2.75	2.28
30	0.033	120	93.1	79	57.9	40.9	21.9	14	8.68	5.17	3.76	2.97	2.47
40	0.025	127	98.9	83.8	61.4	43.3	23.1	14.8	9.16	5.46	3.96	3.13	2.6
50	0.02	133	103	87.7	64.2	45.3	24.1	15.5	9.55	5.68	4.12	3.26	2.7
60	0.017	138	107	90.8	66.5	46.8	24.9	16	9.86	5.86	4.25	3.36	2.79
80	0.013	146	113	95.9	70.1	49.4	26.3	16.8	10.4	6.15	4.46	3.52	2.92
100	0.01	152	118	99.9	73	51.4	27.3	17.4	10.8	6.38	4.62	3.65	3.03
250	0.004	179	138	117	85.1	59.7	31.6	20.1	12.4	7.33	5.3	4.18	3.46

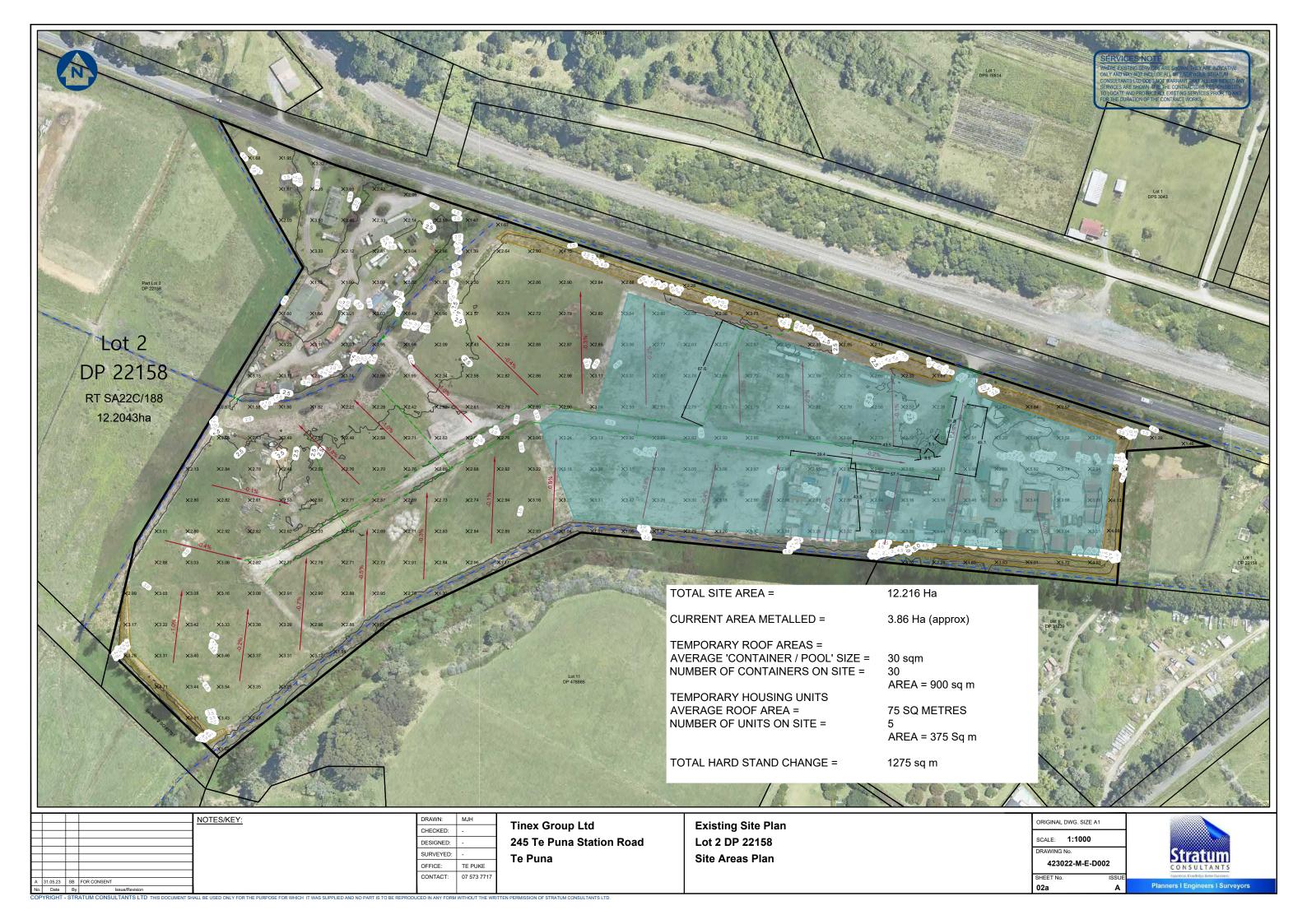
Rainfall intensities (mm/hr) :: RCP8.5 for the period 2031-2050

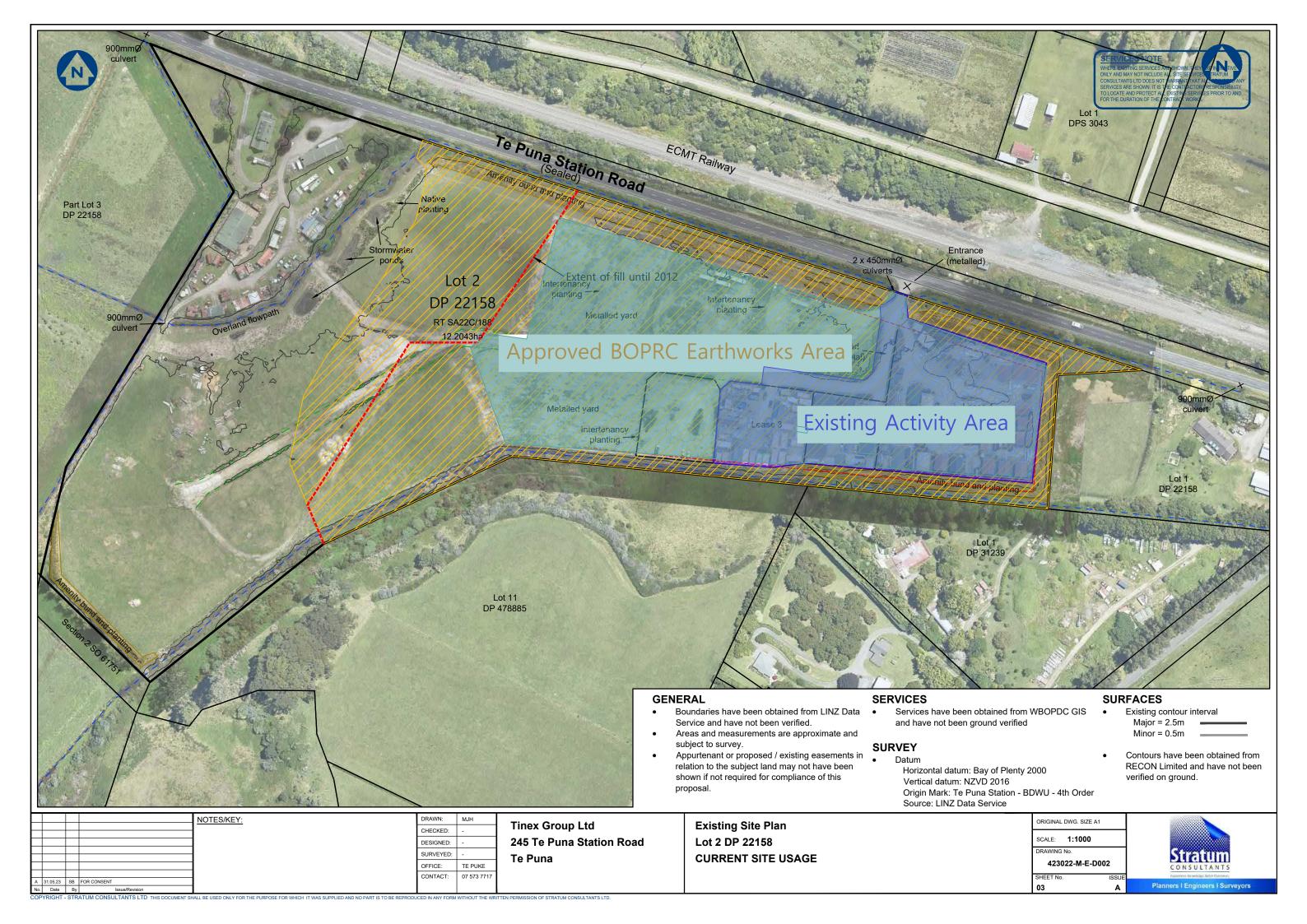
ARI	AEP	10m	20m	30m	1h	2h	6h	12h	24h	48h	72h	96h	120h
1.58	0.633	57	44.8	38.2	28.3	20.1	10.8	6.91	4.29	2.56	1.86	1.47	1.22
2	0.5	63.3	49.7	42.4	31.4	22.3	11.9	7.64	4.72	2.81	2.05	1.62	1.34
5	0.2	85.8	67.1	57.1	42.1	29.8	15.9	10.1	6.24	3.71	2.69	2.12	1.76
10	0.1	103	80.6	68.5	50.4	35.6	18.9	12	7.38	4.38	3.17	2.5	2.07
20	0.05	122	94.9	80.6	59.2	41.7	22	14	8.57	5.07	3.67	2.89	2.39
30	0.033	133	104	88	64.5	45.4	23.9	15.2	9.29	5.49	3.97	3.13	2.59
40	0.025	142	110	93.4	68.4	48.1	25.3	16.1	9.82	5.8	4.19	3.3	2.73
50	0.02	149	115	97.7	71.5	50.3	26.4	16.8	10.2	6.04	4.36	3.43	2.84
60	0.017	154	120	101	74.1	52	27.4	17.3	10.6	6.23	4.5	3.54	2.93
80	0.013	163	126	107	78.2	54.9	28.8	18.2	11.1	6.55	4.72	3.72	3.07
100	0.01	170	132	111	81.4	57.1	30	18.9	11.5	6.79	4.9	3.85	3.18

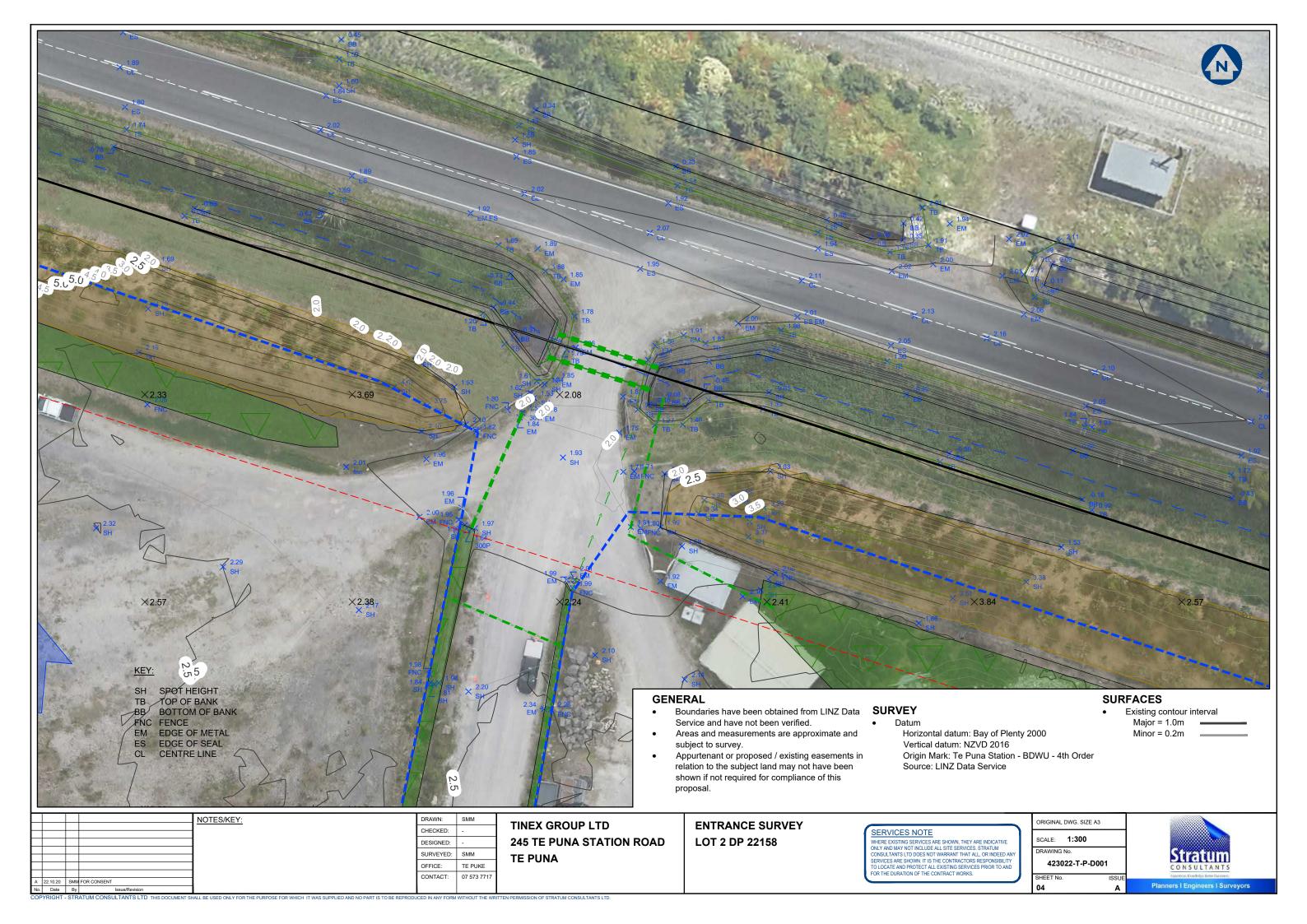
250	0.004	200	154	130	95	66.4	34.7	21.9	13.3	7.79	5.61	4.41	3.64
Rainfall int	ainfall intensities (mm/hr) :: RCP8.5 for the period 2081-2100												
ARI	AEP	10m	20m	30m	1h	2h	6h	12h	24h	48h	72h	96h	120h
1.58	0.633	67.6	53.1	45.4	33.6	23.7	12.4	7.81	4.78	2.8	2.01	1.59	1.31
2	0.5	75.4	59.2	50.5	37.4	26.4	13.8	8.69	5.27	3.1	2.23	1.76	1.45
5	0.2	103	80.5	68.6	50.6	35.6	18.5	11.6	7.03	4.11	2.96	2.32	1.92
10	0.1	124	97	82.5	60.7	42.6	22.1	13.9	8.35	4.88	3.51	2.75	2.27
20	0.05	147	115	97.2	71.4	50	25.9	16.2	9.71	5.67	4.07	3.19	2.62
30	0.033	161	125	106	78	54.5	28.2	17.6	10.5	6.15	4.41	3.45	2.84
40	0.025	171	133	113	82.6	57.8	29.9	18.6	11.2	6.48	4.66	3.64	3
50	0.02	180	139	118	86.5	60.4	31.2	19.4	11.6	6.76	4.84	3.79	3.12
60	0.017	186	145	122	89.6	62.6	32.3	20.1	12	6.98	5.01	3.91	3.22
80	0.013	197	153	130	94.7	66.1	34	21.1	12.6	7.35	5.26	4.11	3.38
100	0.01	206	159	135	98.6	68.7	35.4	22	13.1	7.62	5.45	4.26	3.5
250	0.004	242	187	158	115	79.9	41	25.4	15.1	8.74	6.24	4.88	4.01

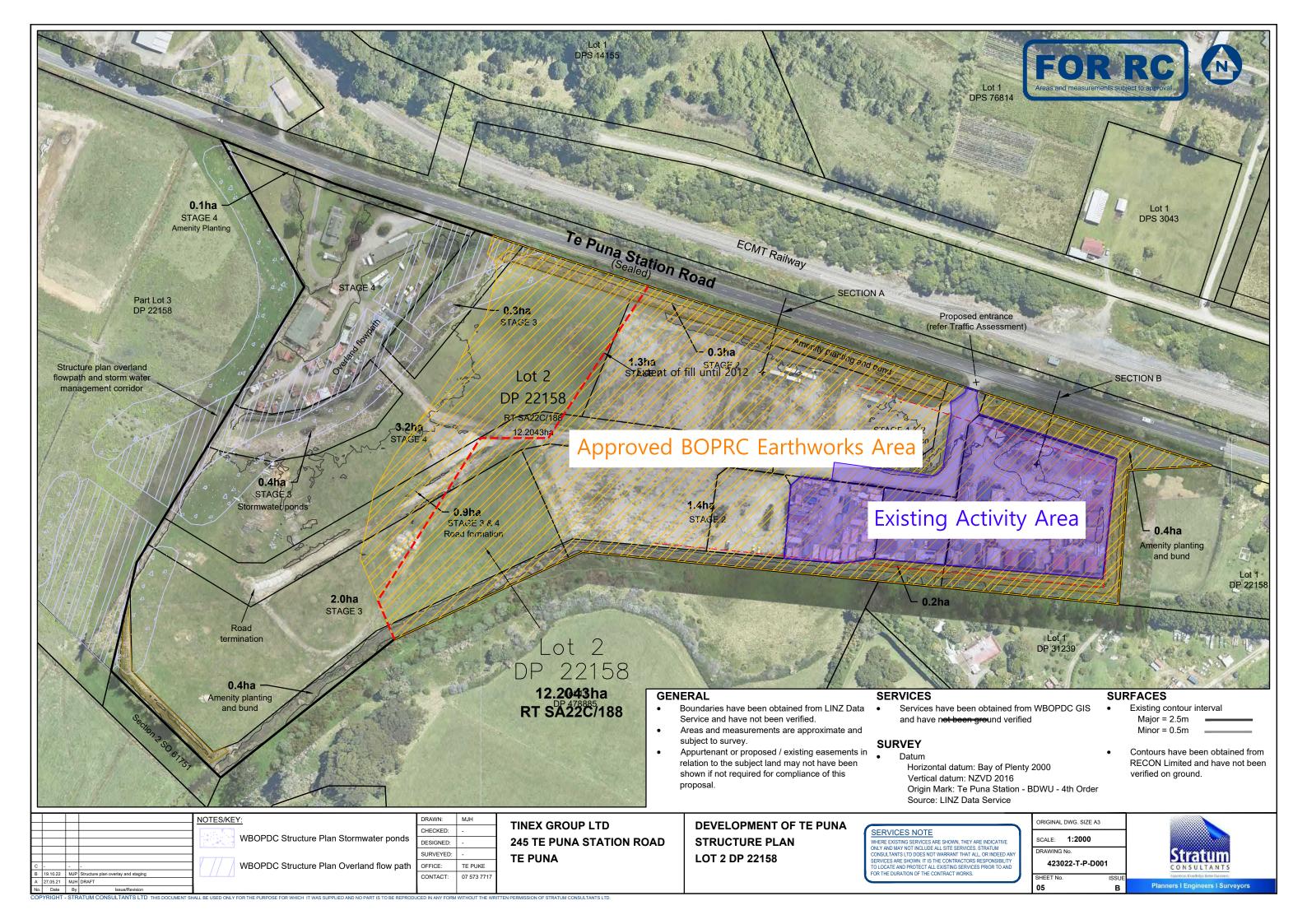








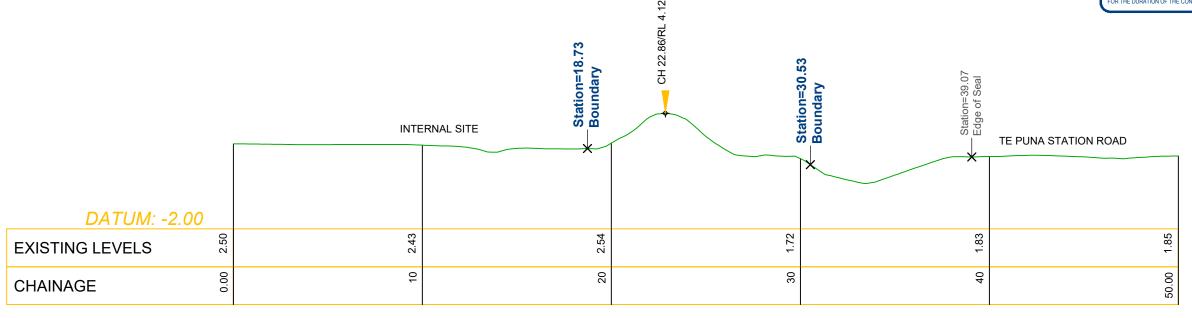




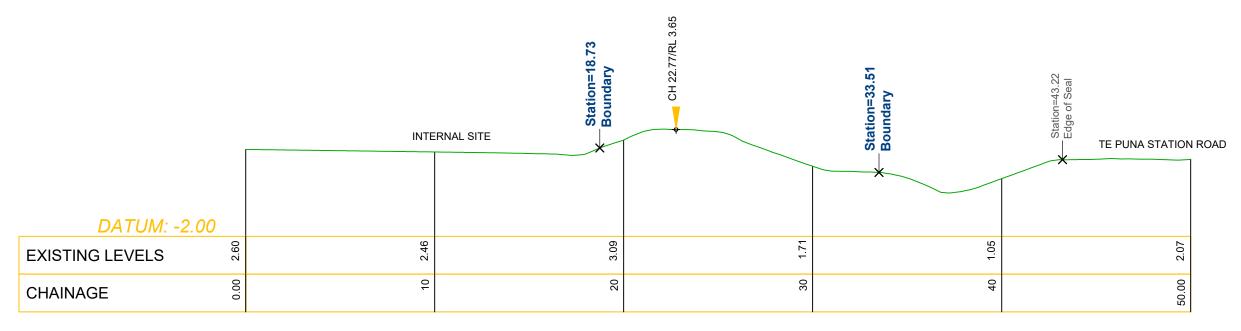


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TO LOCATE AND PROTECT ALL EXISTING SERVICES PRIOR TO AND
FOR THE DURATION OF THE CONTRACT WORKS.



ALIGNMENT A - LONG SECTION



ALIGNMENT B - LONG SECTION

		NOTES/KEY:	DRAWN:	MJH	TINEX GROUP LTD	ORIGINAL DWG. SIZE A3	
			CHECKED:	-		SCALE: 1:2000	
			DESIGNED:	-	245 TE PUNA STATION ROAD		Stratum
		ŀ	SURVEYED:	-	TE PUNA	DRAWING No. 423022-T-P-D001	Stratum
C	- SQ2 resnonse	-	OFFICE:	TE PUKE		423022-1-P-D001	CONSULTANTS
A 27.05.21 MJH DF	DRAFT		CONTACT:	07 573 7717		SHEET No. ISSUE	Planners I Engineers I Surveyors

